

## **APPENDIX B**

### **CALCULATIONS**

- MT13.01** Small Watershed Runoff South Stormwater Pond Watershed
- MT13.02** Small Watershed Runoff Ore Pad
- MT13.03** Ore Pad Cushion Layer Thickness
- MT13.04** Capacity of the Mine Water Treatment Ponds
- MT13.05** North Waste Pile Areas and Volumes



CALCULATION #	MT13.01	PROJECT #	MT13-AC	Page 1 of 2		REV		DATE	
<b>PROJECT:</b>	MT TAYLOR MINE REACTIVATION					0	2/15/2013		
<b>CALCULATION NAME:</b>	Small Watershed Runoff								
	South Stormwater Pond Watershed								
Originator	Alan Kuhn		Checker	Ed Loescher					
<b>Objective:</b>	Calculate the estimated runoff parameters needed to design the capacity of culverts and the south storm water retention pond for events up to the 100 year storm.								
<b>Given:</b>	NPDES #NMR05GB27 requirements apply. Pond capacity must be sufficient to contain design storm runoff. Existing diversions remain.								
<b>Assumptions:</b>	<ol style="list-style-type: none"> <li>1) Discharge of runoff crossing county road south to north will be redirected to south pond. Area north of county road will be drain naturally to Marquez Arroyo.</li> <li>2) New culvert will be added around east and north side of old waste pile.</li> <li>3) For conservatism, pond will be sized to contain all runoff from the 100 year storm, plus up to one additional 100 year storm.</li> </ol>								
<b>References:</b>	NMSHTD Drainage Manual, 1995 NOAA Atlas 14, Vol 1, Version 5, San Mateo Mt Taylor Mine topographic map 2012, by TJ Mann & Associates South Stormwater Pond Watershed Parameters RGR-MT13.AC-CALC-04								
<b>CALCULATION</b>									
<b>Simplified Peak Flow Method</b>	(NMSHTD Drainage Manual, 1995)								
<b>Time of Concentration, Tc</b>	(NMSHTD Drainage Manual, 1995, Vol. 1, eqn 3-18) (South Stormwater Pond Watershed Parameters figure)								
Tc=	$0.0078 * L^{0.77} * S^{(-0.385)}$								
Flow Path		length	slope	elev					
Segment #	1	94	0.02	7500					
	2	119	0.1591						
	3	162	0.1904						
	4	113	0.1005						
	5	65	0.0603						
	6	162	0.0768						
	7	113	0.1557						
	8	67	0.41						
	9	393	0.0032						
	10	251	0.123						
	11	439	0.0262						
	12	339	0.0308						
	13	389	0.012						
	14	551	0.005						
	15	131	0.005						
	16	394	0.005	7304.26					
L, ft = Sum	3782								
S= Ave	0.05176								
Tc=	13.87 minutes								

<b>CALCULATION #</b>	<b>MT13.02</b>	<b>PROJECT #</b>	<b>MT13-AC</b>	<b>Page 1 of 2</b>	<b>REV</b>	<b>DATE</b>
<b>PROJECT:</b>	<b>MT TAYLOR MINE REACTIVATION</b>				0	2/15/2013
<b>CALCULATION NAME:</b>	<b>Small Watershed Runoff</b>		<b>Ore Pad</b>			
Originator	Alan Kuhn	Checker	Ed Loescher			
<b>Objective:</b>	Calculate the estimated runoff parameters needed to design the capacity of culverts and the ore pad runoff retention pond for events up to the 100 year storm, plus antecedent runoff including truck wash.					
<b>Given:</b>	NPDES #NMR05GB27 requirements apply. Pond capacity must be sufficient to contain design storm runoff. Zero discharge from pond required.					
<b>Assumptions:</b>	<ol style="list-style-type: none"> <li>1) All discharge of runoff from the ore pad north will be redirected to ore pad runoff retention pond, formerly the north retention pond.</li> <li>2) Other runoff north of the county road will drain to Marquez Arroyo.</li> <li>3) For conservatism, pond will be sized to contain all runoff from the 100 year storm, plus up to one additional 100 year storm.</li> <li>4) Equal areas with equal slopes drain to each of two catch, then flows combine into pond.</li> <li>5) No separate or added capacity beyond two 100 year storms needed for truck wash runoff.</li> </ol>					
<b>References:</b>	NMSHTD Drainage Manual, 1995 NOAA Atlas 14, Vol 1, Version 5, San Mateo Mt Taylor Mine topographic map 2012, by TJ Mann & Associates					

<b>CALCULATION</b>			
<b>Simplified Peak Flow Method</b> (NMSHTD Drainage Manual, 1995)			
<b>Time of Concentration, Tc</b> (NMSHTD Drainage Manual, 1995, Vol. 1, eqn 3-18)			
Tc= $0.0078 * L^{0.77} * S^{(-0.385)}$			
Flow Path	length	slope	elev
Segment #	1	440	0.01
			7318.4
			7314
L, ft = Sum	440		
S= Ave			0.01
Tc=	4.98 minutes		
<b>Precipitation</b> (NOAA Atlas 14, Vol 1, Version 5, San Mateo)			
In Inches for:			
	Recurrence Interval, years		
Duration	10	50	100
6 hr	1.36	1.84	2.06
24 hr	1.71	2.26	2.51
<b>Area, A</b>	10 acres		
scaled from Mt Taylor Mine topographic map 2012			

<b>CALCULATION #</b>	<b>MT13.02</b>	<b>PROJECT #</b>	<b>MT13-AC</b>	<b>Page 2 of 2</b>	<b>REV</b>	<b>DATE</b>
<b>PROJECT:</b>	<b>MT TAYLOR MINE REACTIVATION</b>				0	2/15/2013
<b>CALCULATION NAME:</b>	<b>Small Watershed Runoff</b>	<b>Ore Pad</b>				

**Runoff Curve Number, CN**

(NMSHTD Drainage Manual,1995, Vol. 1, 3.3.1.3.1; Fig. 3-8, Table 3-1 and 3-4)

Soil Group C

Vegetation type none

Per cent cover <30%

CN = 85 to 91, = 91

**Unit Peak Discharge, qu**

(NMSHTD Drainage Manual,1995, Vol. 1, eqn 3-22)

$qu = 0.543 Tc^{-0.812} = 0.147$  cfs/ac-in

**Direct runoff, Qd**

(NMSHTD Drainage Manual,1995, Vol. 1, eqn 3-23)

$Qd = (P - (200/CN) + 2)^2 / (P + (800/CN) - 8)$

10 yr Qd= 0.91 inches

50 yr Qd= 1.39 inches

100 yr Qd= 1.62 inches

**Peak Discharge, Qp**

$Qp = A * qu * Qd$  to each catch basin

10 yr Qp= 1.35 cfs

50 yr Qp= 2.05 cfs

100 yr Qp= 2.39 cfs

**Runoff Volume, Qv**

$Qv = Qd * A / 12$  to each catch basin total

10 yr Qv= 0.76 ac ft 1.52 ac ft

50 yr Qv= 1.16 ac ft 2.32 ac ft

100 yr Qv= 1.35 ac ft 2.70 ac ft

<b>CALCULATION #</b>	<b>MT13.03</b>	<b>PROJECT #</b>	<b>MT13-AC</b>	Page: 2 of 2	<b>REV</b>	<b>DATE</b>
<b>PROJECT:</b>					0	2/19/2013
<b>CALCULATION NAME:</b>						

**CALCULATION**

Calculate the maximum load on the liner versus allowable load based on puncture resistance, using several cushion layer depths to find necessary depth

Load imposed by 988 loader

- L, Load per tire = 28757 lbs. Ref.1
- A, Tire gross contact area = 652 in.^2 Ref.2
- R, radius of contact area, inches = 14.4 in.
- P, Pressure applied on ground =  $L/A =$  44.11 psi
- Tire width = 35.5 in. Ref.2
- Puncture Resistance of 60 mil HDPE, min. ave, 108 lbs Ref.3  
based on ASTM D6241 using 2.0 in. diameter probe, or 34.4 psi
- Load at liner level Ref. 4

**1) For distributed load Bousinnesq eqn for load vertically below center of tire,  $S_z$**

$$S_z = P * (1 - (1 / (1 + (z/R)^2)))^{1.5}$$

Ref.4, eqn 40.3

z= depth below surface to liner, inches

z	$S_z$ , psi
6	41.60
12	32.53
18	23.11
24	16.30
30	11.79

or:

**2) Alternative Method per Ref. 5 with Froehlich concentration factor**

$$S_z = P [1 - (z / (R^2 + z^2)^{0.5})^v]$$

- $S_z$  = vertical stress at depth z, psi
- P = vertical stress (pressure) at ground surface, psi = 44.11
- z = vertical distance between point load and depth of interest, inches
- R = radius of applied (assumed) circular loaded area, inches = 14.4  
for equivalent circular area of contact for 988 loader
- v = Froehlich concentration factor, for normal soil = 4

$$S_z = 44.11 * [1 - (z / (14.4^2 + z^2)^{0.5})^4]$$

z, inches	$S_z$ , psi
6	43.14
12	36.70
18	27.71
24	20.26
30	14.97

Results:

Method 2 gives the more conservative results.  
Load from equipment on liner <= 34.4 psi at 18 inches, so 2.5 ft (30 in.) of cushion layer+ 12 inches travel course allows for some additional load from soil weight and other unknowns.

<b>CALCULATION #</b>	<b>MT13.03</b>	<b>PROJECT #</b>	<b>MT13-AC</b>	Page: 1 of 2	<b>REV</b>	<b>DATE</b>
<b>PROJECT:</b>	<b>MT TAYLOR MINE REACTIVATION</b>				0	2/19/2013
<b>CALCULATION NAME:</b>	<b>Ore Pad Cushion Layer Thickness</b>					
<b>Originator</b>	Alan Kuhn	<b>Checker</b>	Ed Loescher			
<b>Objective:</b>	Calculate the thickness of soil cushion layer required to protect the underlying HDPE membrane from puncture from equipment working on the pad surface.					
<b>Given:</b>	Location of the ore pad remains as previous. Cushioning material will be locally available SC, CL, SP, and SP-SM soils. A 1.0 ft travel/ drainage layer of gravel or crushed sandstone will overlie the cushion layer.					
<b>Assumptions:</b>	<ol style="list-style-type: none"> <li>1) Maximum load is from a CAT 988 loader</li> <li>2) Cushion layer placed and compacted without specified density, so 85 pcf dry density assumed</li> <li>3) 988 tires are 35/65-33 per CAT manual</li> <li>4) Cushion soil mass is elastic, isotropic, homogeneous and semi-infinite.</li> <li>5) Puncture is primary liner failure mode</li> <li>6) Boussinesq pressure distribution through soil from tire load</li> </ol>					
<b>References:</b>	<ol style="list-style-type: none"> <li>1) CAT Manual 2012</li> <li>2) <a href="http://www.goodyearotr.com/cfm/web/otr/tire-selector/detailresults.cfm?tireid=1557">http://www.goodyearotr.com/cfm/web/otr/tire-selector/detailresults.cfm?tireid=1557</a></li> <li>3) GRI Test Method GM13* Standard Specification for "Test Methods, Test Properties and Testing Frequency for High Density Polyethylene (HDPE) Smooth and Textured Geomembranes"</li> <li>4) Soil Mechanics in Engineering Practice, 3rd Ed. , 1996, K. Terzaghi, R.B.Pech, and G. Mesri</li> <li>5) Estimating Vertical Stress on Soil subjected to Vehicular Loading, ERDC/CRREL TR-09-2 US Army Corps of Engineers, February 2009</li> </ol>					

<b>CALCULATION #</b>	<b>MT13.04</b>	<b>PROJECT #</b>	<b>MT13-AC</b>	<b>REV</b>	<b>DATE</b>
<b>PROJECT:</b>	<b>MT TAYLOR MINE REACTIVATION</b>			0	2/27/2013
<b>CALCULATION NAME:</b>	<b>Capacity of the Mine Water Treatment Ponds</b>				
Originator	Ed Loescher	Checker	Alan Kuhn		
<b>Objective:</b>	Calculate the volume capacity of the mine water treatment ponds. Compare these values with those shown in the 1981 mine water discharge drawings				
<b>Given:</b>	Data from the two referenced mine water flow drawings regarding the water operating levels and areas. The survey data for the levels of the weirs on the hydraulic structures that control the flow in the ponds. New topographic base map from 2012. Some differences are expected due to accumulation of sediments, change to NAD 83 coordinates, and more modern survey and imaging methods.				
<b>Assumptions:</b>	<ol style="list-style-type: none"> <li>1) The minimum water pool elevation (assumes 0 flow through) for the ponds is determined by the elevation of the outlet weirs.</li> <li>2) The maximum pool elevation determined by assuming a 2' freeboard from the top of the pond berms.</li> <li>3) Volumes for the <i>existing</i> (as-is) conditions (Table 1) before upgrades (slope regrade or deepening).</li> <li>4) Volumes after design upgrades (Table 2) are achieved primarily by balanced cut and fill of the pond slopes.</li> </ol>				
<b>References:</b>	<ol style="list-style-type: none"> <li>1) AutoCAD Civil 3d Volume Calculation tools</li> <li>2) Autocad Drawing of Mt Taylor Mine topographic map 2012, by TJ Mann &amp; Associates</li> <li>3) Drawing # 0000-P-983 Titled -"Water Treatment System Flow Schematic Minewater Discharge (Figure IV)- Rev 5 - Dated 9/22/1981</li> <li>4) Drawing # 0000-P-797 Titled -"Water Treatment System Flow Scheme - Rev 4 - Dated 3/06/1981</li> </ol>				

**CALCULATION**

**Methods:** Use Autocad Civil 3d (2013) volume analysis tools to calculate existing volumes, based on existing topography as provided by recent survey by Thomas Mann and Associates, 2012.

<b>Table 1 Ponds in Existing Condition</b>				<b>Table 2 Ponds With Design Upgrades</b>						
	<b>Ref. 3</b>		<b>Ref 4</b>	<b>Based on 2012 Topography and AutoCAD (Ref. 2 and 1)</b>						
<b>Pond Number</b>	<b>Pond Water Level Elevation</b>	<b>Pond Area Sq Ft.</b>	<b>Volume Capacity (Acre Feet)</b>	<b>Pond Number</b>	<b>Operating Pool Elevation</b>	<b>OPL</b>	<b>Area , ft^2</b>	<b>Volume, cy</b>	<b>Volume (acre feet)</b>	
1	7303.4	44600	8.36	<b>1</b>	Min	7305	68938	32911.6	15.32	
					Max	7308		24718	20.40	
2	7297.7	31550	5.88	<b>2</b>	Min	7299.6	31655	5803	3.60	
					Max	7301		7470	4.63	
3	7294.2	43100	8.45	<b>3</b>	Min	7296.7	40373	12420	7.70	
					Max	7300		18130	11.24	
4	7286.4	68750	13.77	<b>4</b>	Min	7287.5	60195	12680.4	7.86	
					Max	7291		14977.3	9.28	
5	7283.0	66000	13.22	<b>5</b>	Min	7285	60218	11728.7	7.27	
					Max	7286		14041	8.70	
6	7282.8	11100	1.72	<b>6</b>	Min	7282.8	6636	698	0.43	
					Max	7286		1622.8	1.01	
7	7282.8	10700	1.72	<b>7</b>	Min	7282.8	6634	697.8	0.43	
					Max	7286		1614.8	1.00	
8	7287.4	45250	9.1	<b>8</b>	Min	7287.5	34105	4489.2	2.78	
					Max	7291		9568.5	5.93	
<b>TOTALS</b>	<b>AREA</b>	<b>321050</b>		<b>308754</b>						
	<b>VOLUME</b>		<b>62.22</b>	<b>VOLUMES AT MIN</b>						<b>50.47</b>
				<b>VOLUMES AT MAX</b>						<b>57.11</b>



<b>CALCULATION #</b>	<b>MT13.05</b>	<b>PROJECT #</b>	<b>MT13-AC</b>	<b>REV</b>	<b>DATE</b>
<b>PROJECT:</b>	<b>MT TAYLOR MINE REACTIVATION</b>			0	3/24/2013
<b>CALCULATION NAME:</b>	<b>North Waste Pile Areas and Volumes</b>				
Originator	Ed Loescher	Checker	Alan Kuhn		
<b>Objective:</b>					
Calculate various earthwork quantities for the CCP for the future North Waste Rock Pile.					
<b>Given:</b>					
The basic design of the north waste rock pile with connected runoff retention basin as shown in Drawing MT13-AC-10. Base topography from 2012 Thomas Mann survey.					
<b>Assumptions:</b>					
1) Quantities shown are for final build out					
2) Retention basin and surrounding diversion channel earthwork are separate from the main pile and will be performed first.					
<b>References:</b>					
1) Drawing MT13-AC-10					
2) AutoCAD Volume Calculation method					
<b>CALCULATION</b>					
AutoCAD Civil 3D 2012 used to find areas and volumes, applying base topography compared to the final topography at full buildout.					
<b>North Waste Rock Retention Runoff Basin</b>					
Basic design parameters				Reference #1	
Top of basin berm	7284	feet			
Bottom of basin excavation	7272	feet			
Basin side slopes	4H: 1V				
High water elevation	7282	feet			
Surface area for clay liner	80097	Sq ft			
Volume of clay liner, 1 ft thick	2967	CY			
Capacity at high water level	22.1	Acre Feet			
<b>Earthwork quantities</b>					
Cut for basin	18291	CY			Reference #2
Fill for basin embankment	4792	CY			
Net clean soil to be stockpiled for cover	13499	CY			
<b>North Waste Rock Pile (at full buildout)</b>					
Dimensions (maximum)				Reference #1	
Top of pile	7352	feet			
Bottom of pile	existing grades				
Pile side slopes	5H: 1V				
<b>Calculated Areas and Volumes</b>					
				Reference #2	
Total area of waste rock pile cover	805372	Sq Ft			
Total volume of waste rock pile cover	59,657	CY			
Total area of the top of the pile	274071	Sq Ft			
Total slope areas	531301	Sq Ft	12.20		Acres
Total disturbed area	1184398	Sq Ft	27.19		Acres
<b>Earthwork quantities</b>					
Fill capacity for waste rock storage at full buildout	675525	CY			
Cut for diversion channel around pile	8278	CY			
Clean fill for containment berm around pile	13474	CY			

CALCULATION #	MT13.06	PROJECT #	MT13-AC	Page	1 of 3	REV	DATE
<b>PROJECT:</b> MT TAYLOR MINE REACTIVATION						0	3/25/2013
<b>CALCULATION NAME:</b> South Waste Pile and Retention Pond Areas and Volumes							
<b>Originator</b> Ed Loescher		<b>Checker</b> Alan Kuhn					
<b>Objective:</b> Calculate various earthwork quantities for the CCP for the South Waste Rock Pile and South Storm water retention pond.							
<b>Given:</b> The basic design of the south waste rock pile and south storm water retention pond as shown in Drawings MT13-AC-08 and MT13-AC-16. Base topography from 2012 Thomas Mann survey.							
<b>Assumptions:</b>							
1) Quantities shown are for final build out and for the initial reshaping of the pond and pile							
2) Retention basin excavation and excavation of the SW Corner of the existing pile are primarily in clean soil deposits and will be treated separately. The first phase of earthwork will occur as part of the mine reactivation. This will include excavation and expansion of the South Storm Water Retention Basin and Reshaping the slopes of the existing south Waste Rock pile. See Reference #1.							
3) The final buildout will be placed on top of the phase 1 surface. This assumes a level surface at elevation 7347 ft. See Reference #1.							
<b>References:</b>							
1) Drawing MT13-AC-08 and MT13-AC-16							
2) AutoCAD Volume Calculation method							
3) Attached Drawing MT13-Calc-13.06							
<b>CALCULATION Mine Reactivation</b>							
AutoCAD Civil 3D 2012 used to find areas and volumes, applying base topography compared to the final topography at full buildout.							
<b>South Storm Water Retention Basin</b>							
Basic design parameters						Reference #1	
Top of basin berm				7304	feet	Reference #3	
Bottom of basin excavation				7290	feet		
Basin side slopes				4H: 1V			
High water elevation				7302	feet		
Surface area for clay liner				86660	Sq ft		
Volume of clay liner, 1 ft thick				3210	CY		
Capacity at high water level				15.4	Acre Feet		
<b>Earthwork quantities</b>							
Cut for basin (Clean Soil)				22567	CY	Reference #2	
Cut for basin (Pond Sediments)				6776	CY	reference #3	
Fill for basin embankment				4792	CY		
Net clean soil to be stockpiled for cover				17775	CY		

<b>CALCULATION #</b> MT13.06	<b>PROJECT #</b> MT13-AC	<b>Page</b> 2 of 3	<b>REV</b> 0	<b>DATE</b> 3/25/2013
<b>PROJECT:</b> MT TAYLOR MINE REACTIVATION				
<b>CALCULATION NAME:</b> South Waste Pile and Retention Pond Areas and Volumes				

**Reshaping the South Waste Rock Pile (at first phase earthwork) See Assumption #3.**

Elevations  
Top of pile 7247 feet Reference #1  
Bottom of pile existing grades reference #3  
Pile side slopes 5H: 1V

**Calculated Areas and Volumes** Reference #2

Total area of waste rock pile cover	246240	Sq Ft		
Total volume of waste rock pile cover	18,240	CY		
Total slope areas (in waste rock area)	246240	Sq Ft	5.65	Acres
Revegetated Area (excludes the area of the pond with clay liner, and the top of the waste rock pile)	622569	Sq Ft	14.29	Acres

**Earthwork quantities**

**Fill Deposited in Waste Rock Pile From Various Sources**

Mine Water Treatment Pond Sediments (ponds 1-8, Area A, And Ore Pad Retention Basin)	18310	CY
Ore Pad Soil Cleanup	5000	CY
Sediments from South Storm Water Retention Basin	6776	CY
Waste Rock cut from re-shaped slopes	47490	CY
<b>Total Fill Soils</b>	<b>77576</b>	<b>CY</b>

**Reshaping the SW Corner of the South Waste Rock Pile (at first phase earthwork) See Assumption #3.**

Elevations  
Top of pile 7247 feet Reference #1  
Bottom of pile existing grades reference #3  
Pile side slopes 5H: 1V

**Calculated Areas and Volumes** Reference #2

Total slope areas (in Clean Soil area)	55316	Sq Ft	1.27	Acres
Total slope areas (in New Clean Soil Stockpile)	38200	Sq Ft	0.88	Acres

**Earthwork quantities**

**Cut soils in SW Corner of Waste Rock Pile (Clean Soils)**

Total Cut soils from Clean Soil Area	48122	CY
Fill needed in SW Corner	3660	CY
Clean Cover Soils needed in Waste Rock Area	18240	CY
<b>Net Clean soils to New Stockpile</b>	<b>26222</b>	<b>CY</b>

<b>CALCULATION #</b> MT13.06	<b>PROJECT #</b> MT13-AC	<b>Page</b> 3 of 3	<b>REV</b>	<b>DATE</b>
<b>PROJECT:</b> MT TAYLOR MINE REACTIVATION			0	3/25/2013
<b>CALCULATION NAME:</b> South Waste Pile and Retention Pond Areas and Volumes				
<b>CALCULATION Final Closeout</b>				
<b>South Waste Rock Pile (at full buildout) Assumption #4</b>			Reference #3	
Elevations			Reference #1	
Top of pile elevation	7372	feet		
Bottom of pile (top of Phase 1 pile)	7347	feet		
Pile side slopes	5H: 1V			
<b>Calculated Areas and Volumes</b>			Reference #2	
Total new surface area	361386	Sq Ft	8.30	Acres
Total new top area	121860	Sq Ft	2.80	Acres
Total new slope areas (excludes the work done previously)	239526	Sq Ft	5.50	Acres
<b>Earthwork quantities</b>				
Fill capacity for waste rock storage at full buildout	178760	CY		
2 Feet of Clean Soil Cover	26770	CY		

SEGMENT NUMBER	STARTING ELEVATION	SEGMENT LENGTH FEET	SLOPE PERCENT	SURFACE DESCRIPTION
1	7500	94.14	2.00%	Rocky Mountain side
2	7498	119.13	-15.91%	steep cliff
3	7479	161.5	-19.04%	steep cliff
4	7448	112.6	-10.05%	Arroyo
5	7437	64.5	-6.03%	Arroyo
6	7433	162.4	-7.68%	gravel road way
7	7421	113.1	-15.57%	gravel road way
8	7403	67	-41.00%	steep cliff
9	7376	393.3	-0.32%	gravel road way
10	7374	251	-12.30%	gravel road way
11	7344	438.6	-2.62%	gravel road way
12	7332	339	-3.08%	drainage ditch

WATER ENTERS STORM SYSTEM

13	7314.3	66.4	-1.20%	MH 5 - 48' CMP
14	7313.49	322.3	-1.20%	MH 4 - 48' CMP
15	7309.64	550.7	0.50%	MH 3 - 48' CMP
16	7306.88	130.7	0.50%	MH 2 - 48' CMP
17	7306.23	394.4	0.50%	MH 1 - 48' CMP

TOTAL AREA = 77.5 ACRES

CALCULATION PURPOSE: PROVIDE BACKGROUND DATA FOR SOUTH STORM WATER RETENTION POND DESIGN. DATA REQUIRED IS CONTRIBUTING AREAS IN ACRES AND LENGTHS WITH SLOPES FOR THE VARIOUS SEGMENTS OF THE FLOW PATH.

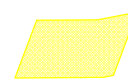
CALCULATION PROCEDURE: FROM AUTOCAD CIVIL 3D DETERMINE THE FLOW LINE FROM THE MOST UPSTREAM POINT TO THE POINT OF DISCHARGE AT THE POND. CREATE A FEATURE LINE FOLLOWING THE FLOW PATH. FIND THE LENGTHS AND SLOPES OF EACH SEGMENT BY LISTING THE SEGMENT LINE PROPERTIES IN AUTOCAD.

CALCULATION DOCUMENTS: 3D TOPOGRAPHY FROM AERIAL SURVEY DRAWING. THOMAS MANN, SURVEYOR 2012.

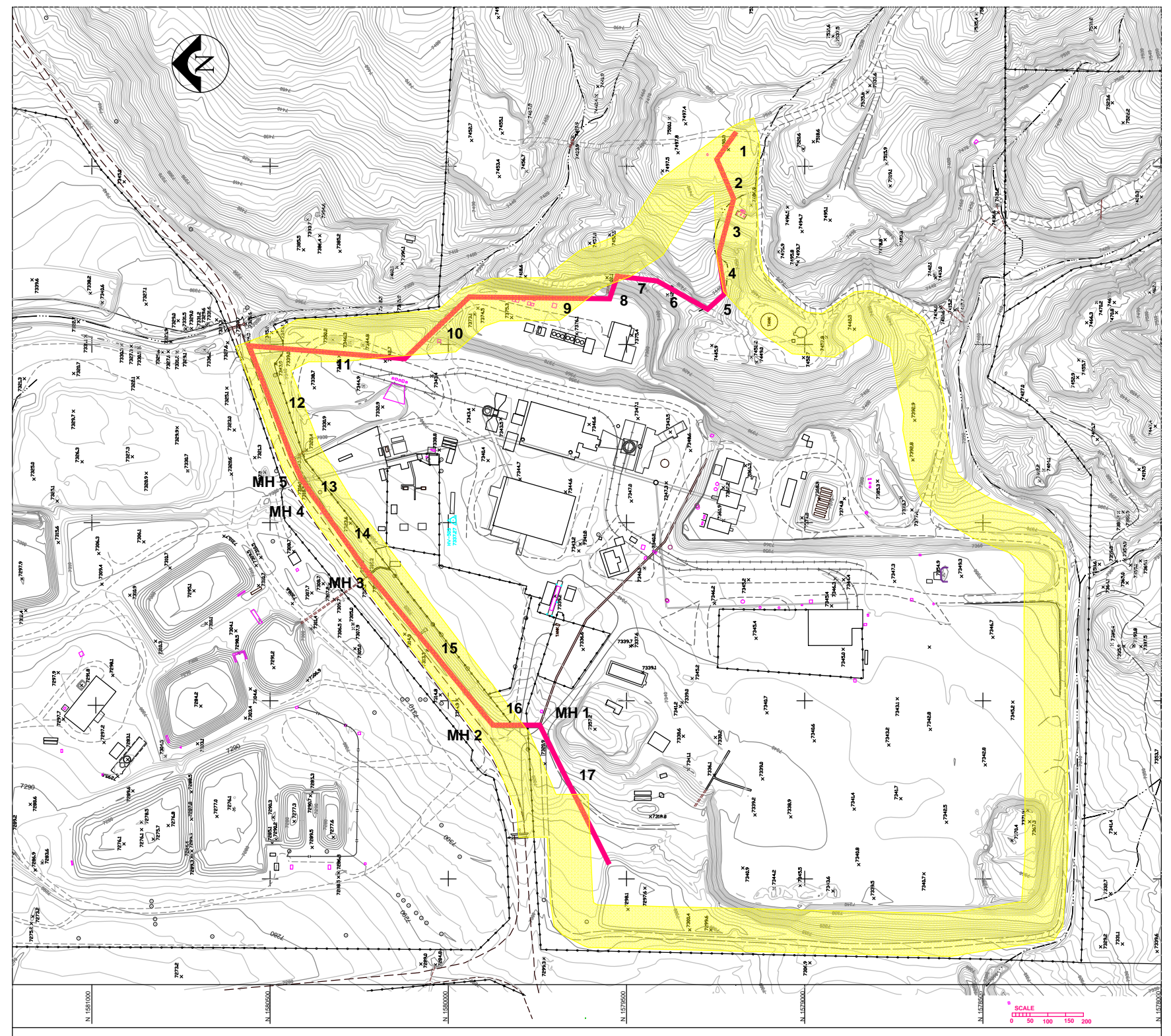
### LEGEND



FLOW LINE



CONTRIBUTING AREA = 77.5 ACRES



REV	DESCRIPTION	DATE	DRAWN BY	ENGINEER	APPROVED	<b>RIO GRANDE RESOURCES CORPORATION</b> MOUNT TAYLOR MINE - Grants, NM 87020		PROJECT TITLE:	REV	
A	2013 CONTRIBUTING AREA	2/4/13	EL	AK		Prepared By:	SIZE <b>B</b>	SCALE: AS NOTED	SHEET TITLE:	
						Alan Kuhn Associates LLC	CALC NO.	RGR-MT- AC-CALC 01	<b>SOUTH STORM WATER POND CALCULATION FIGURE 1</b>	<b>A</b>